

River Engineering

Tutorial Sheet 1 – Resistance laws for steady uniform flow

1. Gauckler-Manning-Strickler

- Plot a graph of Strickler's relationship $n = D^{1/6} / (6.7\sqrt{g})$ for D from 0.1 mm to 30 cm (logarithmic D axis, linear n axis). Keep for future use?
- Compare the two values you get for $D = 1$ cm and 2 cm. Comment.
- Do you have an explanation for this surprising insensitivity of n to changes in boundary grain size? (If you do, please tell the lecturer. We could discuss it.).

2. For a trapezoidal cross-section, where the bottom width is W , the depth is h , and the side slopes are (H:V) $m : 1$,

- Show the following properties:

$$\text{Top width } B = W + 2mh$$

$$\text{Area } A = h(W + mh)$$

$$\text{Wetted perimeter } P = W + 2\sqrt{1 + m^2}h$$

$$\text{First moment about surface } A\bar{h} = \frac{1}{2}Wh^2 + \frac{1}{3}mh^3$$

(use the fact that the centroid of a triangle is 1/3 of the distance from the base).

- Verify that the general relationship $dA/dh = B$ holds true for this section.
 - Verify the general relationship $d(A\bar{h})/dh = A$ also holds.
3. Use the Gauckler-Manning formulation and the results for a trapezoidal section to obtain an expression for the steady uniform discharge as a function of depth h :

$$Q = \frac{1}{n} \frac{(h(W + mh))^{5/3}}{(W + 2h\sqrt{1 + m^2})^{2/3}} \sqrt{S_0} \quad (1)$$

4. A commonly-encountered problem in river and canal hydraulics is to compute the normal (uniform flow) depth for a given discharge by numerically solving the transcendental equation you obtained in question 3. Here we consider methods for solving the problem:

- A traditional method is to plot the relationship and read the solution off. However, this is not suited to computer programs.
- Trial and error – substitute numerical values of h in until the required discharge is obtained, also not suited to computer programs.
- Solve the equation by a numerical method for solving transcendental equations such as direct iteration, bisection, or Newton's method, or most simply, using Solver in Excel.
- The direct iteration scheme in the lectures works well:

$$h_0 = \left(\frac{nQ}{B_0\sqrt{S}} \right)^{3/5}$$

$$h_{n+1} = \left(\frac{nQ}{\sqrt{S}} \right)^{3/5} \times \frac{P^{2/5}(h_n)}{A(h_n)/h_n} = \left(\frac{nQ}{\sqrt{S_0}} \right)^{3/5} \frac{(W + 2h_n\sqrt{1 + m^2})^{2/5}}{W + mh_n},$$

which is then evaluated repeatedly.

- e. Obtain the corresponding iteration scheme for the Chézy-Weisbach formulation.

$$h_{n+1} = \left(\frac{\lambda Q^2}{8gS_0} \right)^{1/3} \frac{P^{1/3}(h_n)}{A(h_n)/h_n} = \left(\frac{\lambda Q^2}{8gS_0} \right)^{1/3} \frac{(W + 2h_n\sqrt{1+m^2})^{1/3}}{W + mh_n}.$$

- f. Check: is that dimensionally correct? *Always* check for dimensional correctness.
- g. Note that such direct iteration does not work for all equations – it did work here because we made sure that our equation was a weakly varying function of h .
5. Consider a canal with $W = 10$ m, side slopes of 2:1, a longitudinal slope of 0.0005, $n = 0.02$. For a flow of $20 \text{ m}^3 \text{ s}^{-1}$, calculate the normal depth h .
- a. If you have access to a spreadsheet with Solver installed, use that to solve the problem.
- b. Use the direct iteration scheme in question 4.d
 (Ans: $h = 1.348$ m – if we start with $B_0 = W = 10$, we then obtain $h_0 = 1.418$, $h_1 = 1.343$, $h_2 = 1.349$, $h_3 = 1.348$ m, and it has converged to within a millimetre).